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Advanced Experimental Evaluation of Asphalt Mortar for Induction Healing Purposes

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Advanced Experimental Evaluation of Asphalt Mortar for Induction Healing Purposes

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ABSTRACT

This paper studied the induction heating and healing capacity of asphalt mortar by adding electrically conductive additives (e.g. iron powder and steel fibers), and examined the influence of different combinations of them on the mechanical response of asphalt mortars. Induction heating technique is this innovative asphalt pavement maintenance method that is applied to the conductive asphalt concrete mixtures in order to prevent the formation of macro-cracks by increasing locally the temperature of asphalt mixtures. It was found that increasing steel fiber content within the asphalt mortar the tensile strength and the fatigue life increased respectively. It was also proved that the conductive asphalt mortars with iron powder appeared improved mechanical response when steel fibers were added. Furthermore, it was observed that asphalt mortars containing a combination of additives – steel fibers and iron powder - demonstrate a better induction heating efficiency than mortars including only steel fibers. Finally, the induction healing capacity of conductive asphalt mortars is determined.

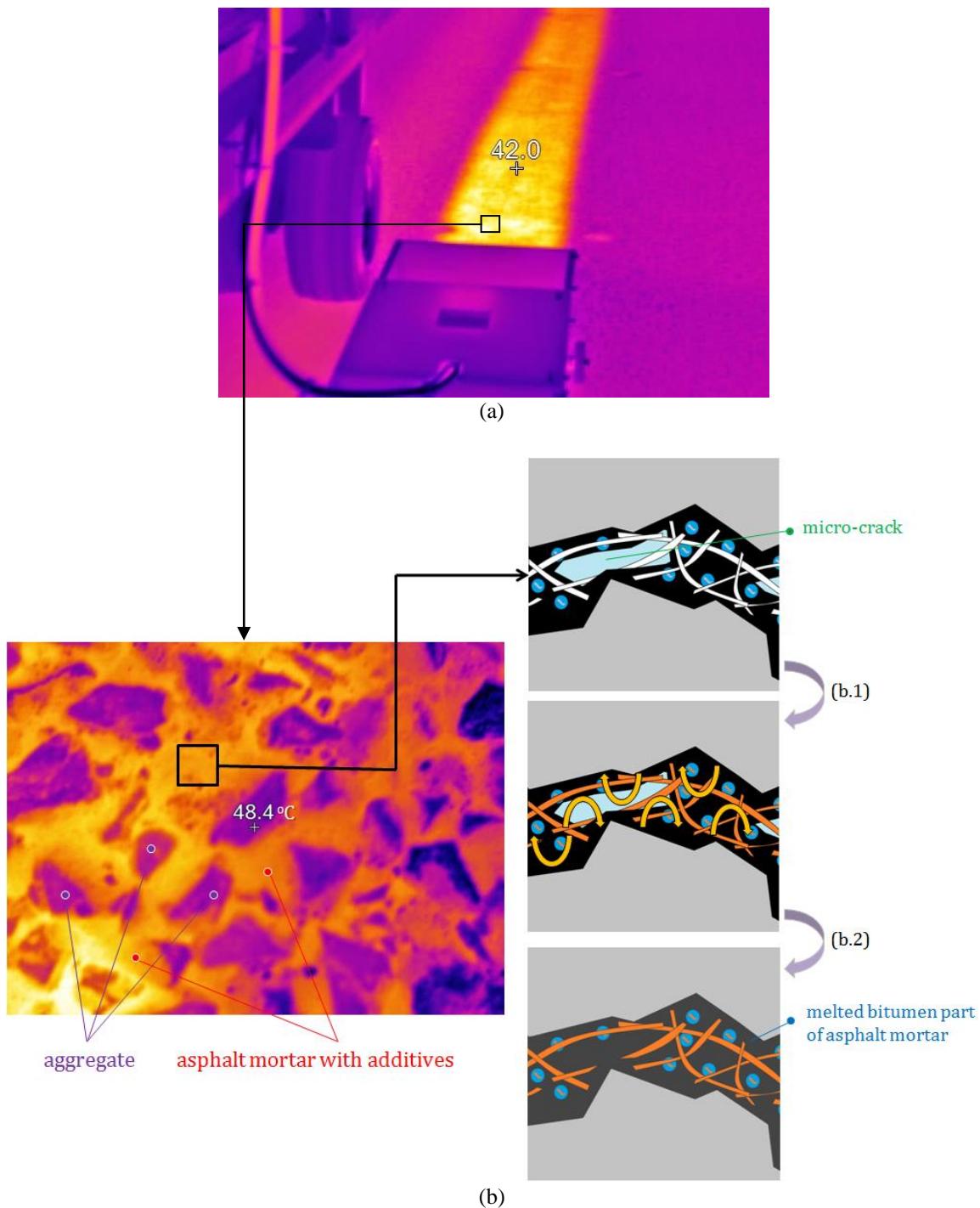
1 INTRODUCTION

2 Asphalt concrete mixtures are the most common types of pavement surface materials applied in transportation
3 infrastructures and consist of asphalt binder, aggregate particles and air voids. These mixtures are
4 temperature-dependent materials with self healing capability because they can restore stiffness and strength
5 (1-3). Because of the importance of reducing the energy consumption and the corresponding emissions of
6 CO₂, many investigations of new materials with enhanced functionalities have taken place recently.
7 Moreover, the necessity of developing more durable and sustainable pavement structures has led the
8 pavement industry to search for new ways of solving construction and rehabilitation issues. Hence, the
9 employment of state of the art techniques, for construction or maintenance is becoming more and more
10 important.

11 Regarding asphalt pavement maintenance, there are various techniques that can be used to restore the
12 mechanical characteristics of mixtures during their lifespan (4-6). Induction heating technique is one of the
13 promising techniques to prolong the service life of asphalt pavements. Field trials are already available and a
14 very exciting example is the Dutch motorway A58 near Vlissingen, see Figure 1.a. This technique requires
15 new mixtures with conductive additives in order to make them suitable for induction heating. Particularly, the
16 alternating magnetic field induces eddy currents in the additives and consequently heats them according to
17 the principles of Joule's law. The generated heat in the additives increases locally the temperature of the
18 asphalt mixture, through the temperature rise the bitumen is melting, the micro-cracks are healed, see Figure
19 1.b, and the mechanical properties of the pavement are recovered. This approach of introducing induction
20 heating with main purpose to activate the self-healing capacity of porous asphalt is named induction healing.

21 Previous research indicated that asphalt mixtures, with the addition of conductive additives, such as steel
22 fibers, can be heated in a very short time by using the induction heating technology (7-12). However, the
23 distribution of steel fibers within mixtures appears to have a direct relation with the volumetric and
24 mechanical properties (13-20) of asphalt mixtures and it was observed that the characteristics of steel fibers –
25 diameter and length - are affected by the mixing and compaction processes (11). It is very important to
26 develop conductive asphalt mixtures with well dispersed conductive particles to provide sufficient isotropic
27 properties to the materials. For this reason, filler-sized conductive additives can be added into asphalt
28 mixtures as alternatives to study the influence of different combinations of additives on the mechanical
29 response of asphalt mixtures and the induction heating and healing efficiency and the mechanical response of
30 asphalt mixtures.

31 During the induction heating, the asphalt mortar part of asphalt concrete with conductive additives is
32 heated locally without heating the stone aggregates. Thus, asphalt mortar with additives is selected to be
33 investigated in this research. The effect of different volumes of steel fibers and iron powder on the electrical
34 and thermal properties is evaluated by using a digital multimeter and a thermal sensor (CTherm Analyzer),
35 respectively. After the electro-thermal investigation, the tensile strength and fatigue performance of
36 conductive asphalt mortars are studied. As mentioned above, although the reinforcing impact of steel fibers
37 on mechanical properties of asphalt mixtures has been studied extensively, still limited research was ensued
38 to appraise the performance of asphalt mortars with different conductive additives. Furthermore, the
39 induction heating and healing capacity of conductive asphalt mortars is examined as well. The objective of
40 this paper is to study experimentally the structural and non-structural performance of induction heated asphalt
41 mortars since it is the crucial part of asphalt concrete that suffers more damage and contains the conductive
42 additives for induction heating.

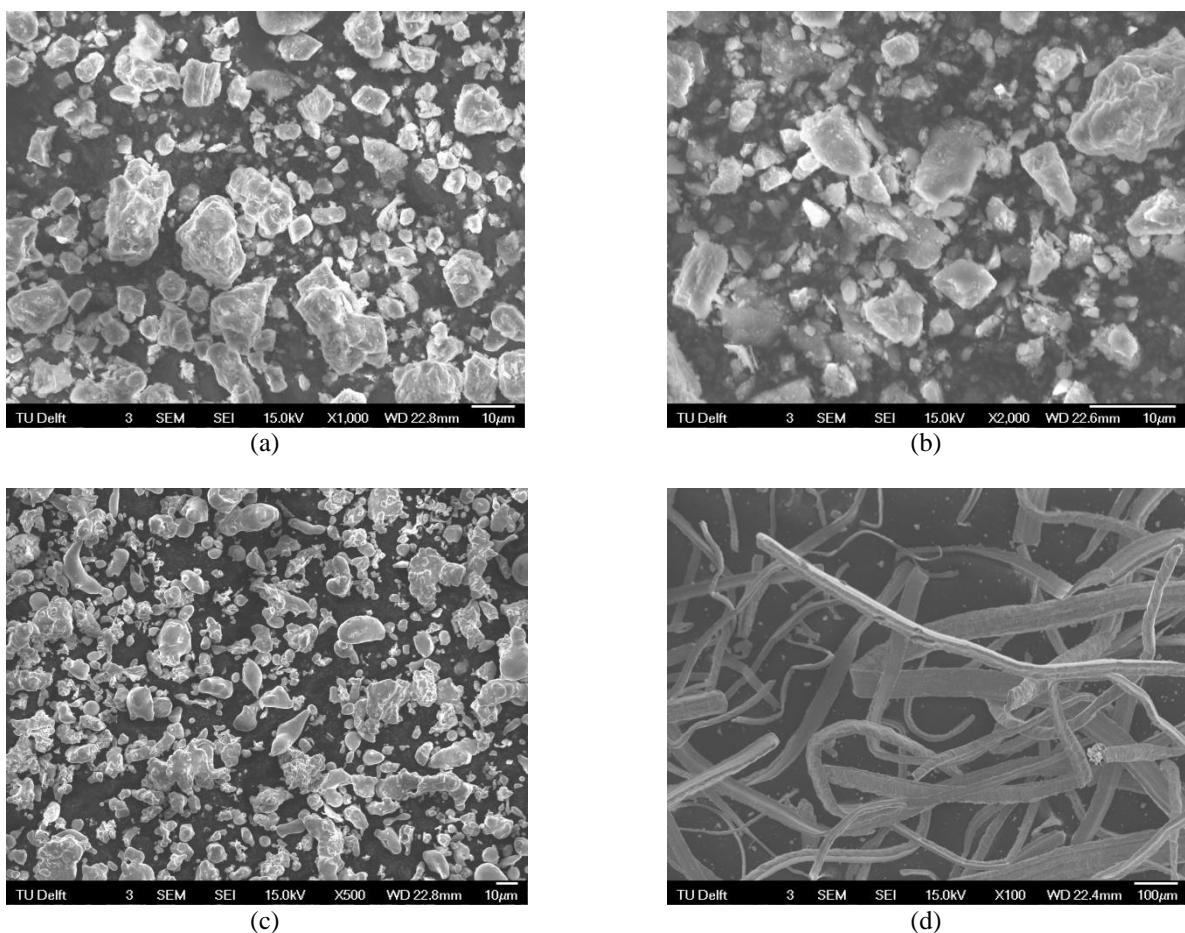


45 **FIGURE 1** Infrared image (a) during induction heating of an asphalt pavement (A58 near Vlissingen,
46 the Netherlands) and (b) of heated asphalt pavement surface at high resolution with the schematic of
47 induction heating, (b.1) asphalt mortar with micro-cracks induced by eddy currents and (b.2) closure
48 of micro-cracks through the heat generation in the asphalt mortar

49 **MATERIAL AND PREPARATION**

50
 51 The original asphalt mortar without electrically conductive additives consists of sand (2697 kg/m³), weak
 52 limestone (WL) filler (2781 kg/m³), produced limestone (PR) filler (2699 kg/m³) and SBS modified bitumen
 53 (1030 kg/m³). The weight percentage of these components in the original asphalt mortar is 33%, 5%, 34%
 54 and 28 % m/m for mineral filler WL, PR, sand and bitumen, respectively.

55 For the development of conductive asphalt mortar, iron powder (7507 kg/m³) was added as a filler-sized
 56 additive after substituting the equivalent volumetric part of mineral fillers - WL mineral filler and PR mineral
 57 filler - in order to avoid volumetric degradation. Figure 2 shows the used different filler-size particles,
 58 mineral and additives, and steel fibers. Steel fibers (7756 kg/m³) are mixed with the other components
 59 without replacing any of them added as a volume percentage of bitumen. In this investigation, the conductive
 60 asphalt mortars are prepared with different volume percentages of iron powder (5%, 10%, 15%, 20% and 25%)
 61 and the amount of steel fiber by volume of bitumen is kept constant (4%). The compositions of the different
 62 conductive asphalt mortars (MA_F()_P()) are given on Table 1. The notation MA indicates asphalt mortar, F
 63 represents filler, P represents iron powder. The values in the brackets indicate the corresponding volume of
 64 the components.
 65



66 **FIGURE 2 SEM SEI images of the filler-size mineral particles: (a) weak limestone (WL) and (b)**
 67 **produced limestone (PR), the conductive additives: (c) iron powder and (d) steel fibers**

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69
70**TABLE 1 Composition of different conductive asphalt mortars**

Type of Asphalt Mortar	Bitumen (% m/m)	Sand (% m/m)	Mineral filler WL (% m/m)	Mineral filler PR (% m/m)	Iron powder (% m/m)
MA_F100_P0	28.00	34.00	33.00	5.00	0.00
MA_F95_P5	28.00	34.00	31.35	4.75	5.15
MA_F90_P10	28.00	34.00	29.70	4.50	10.30
MA_F85_P15	28.00	34.00	28.05	4.25	15.45
MA_F80_P10	28.00	34.00	26.40	4.00	20.60
MA_F75_P25	28.00	34.00	24.75	3.75	25.75

MA: asphalt mortar, F: mineral filler, P: iron powder, steel fiber (volume of bitumen): 4%

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A volume combination of iron powder and steel fibers in the asphalt mortar are determined from the electrical conductivity measurements as reported in this paper and this will be used for the further experimental investigations.

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EXPERIMENTAL METHODS

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Electrical Resistivity and Thermal Conductivity

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After the preparation of the conductive asphalt mortar, the material was poured in silicon-rubber moulds, to obtain samples with rectangular dimensions $125 \times 20 \times 25$ mm. The electrical resistivity measurements were done by performing the two-electrode method, see Figure 3.a, at a room temperature of 20°C . The geometry and the electrical resistivity of the conductive asphalt mortars are the only parameters that influence the resistance. Therefore, the electrical resistivity was obtained from the second Ohm-law:

$$\rho = \frac{RS}{L} \quad (1)$$

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where ρ is the electrical resistivity, measured in Ωmm , L is the internal electrode distance, measured in mm, S is the electrode conductive area measured in mm^2 and R is the measured resistance, in Ω .

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Thermal conductivity measurements were performed by using the C-Therm TCi thermal analyzer, shown in Figure 3.b. The sensor is working according to the Modified Transient Plane Source Method to determine the thermal resistivity and effusivity of the conductive asphalt mortar. The prepared specimen for this test has a diameter of 17 mm to cover the entire sensor. The sensor is heated by a small current and the response is monitoring while in contact with the specimen. The resistivity and effusivity of the specimen were measured and obtained directly from the sensor. From the inverse of the resistivity the conductivity was acquired. Using the effusivity concept other thermal properties like heat capacity and diffusivity can be derived. The effusivity is given by:

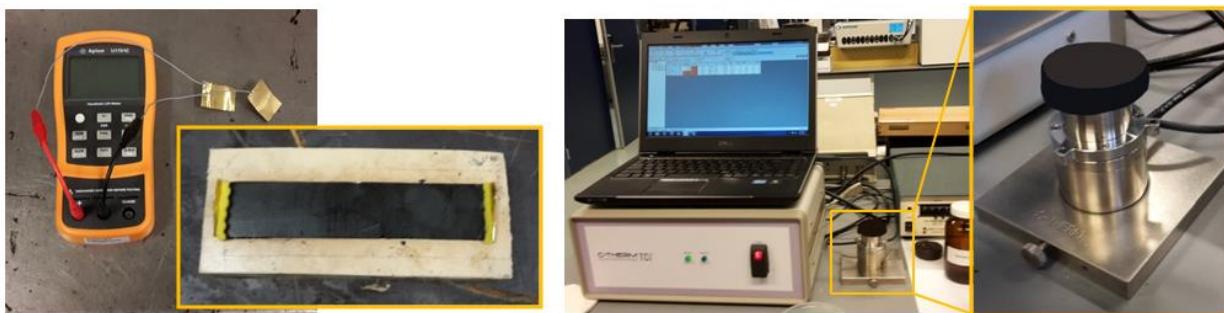
$$\text{Effusivity} = \sqrt{k \cdot \rho \cdot c_p} \quad (2)$$

98

99 where k is the thermal conductivity [$\text{W}/\text{m}\cdot\text{K}$], ρ is the density [kg/m^3] and c_p is the heat capacity [$\text{J}/\text{kg}\cdot\text{K}$].
 100 The thermal conductivity is defined from the Fourier law as:
 101

$$q = -k \cdot \frac{dT}{dx} \quad (3)$$

102
 103 where q is the heat flux (the amount of thermal energy flowing through a unit area per unit time), $\frac{dT}{dx}$ is the
 104 temperature gradient and k is the coefficient of thermal conductivity, often called thermal conductivity. The
 105 heating, reading and cooling process was repeated 6 times per specimen to obtain an average of the reading.
 106



107
 108 **FIGURE 3 (a) Digital multimeter for electrical resistivity measurement and (b) C-Therm TCi thermal**
 109 **analyzer for thermal properties measurements**

110 Direct Tensile Strength and Fatigue Performance

111 In order to investigate the impact of conductive additives on the mechanical properties of the asphalt mortar,
 112 direct monotonic tensile tests are carried out. A 25 kN electro-hydraulic servo testing machine is used, see
 113 Figure 4.a. The monotonic tension tests with freely rotating hinges are performed on specimens from
 114 conductive asphalt mortar, see Figure 4.b. In order to reduce undesired eccentricities, the specimens were
 115 carefully positioned in the special designed steel hinges, see Figure 4.b.1. Furthermore, the conductive
 116 asphalt mortar specimens have a parabolic geometry, with height of 34 mm for the parabolic part and a
 117 thickness of 10 mm in the middle. The monotonic tension tests were performed at different displacement
 118 rates. The fatigue performance is tested in load control mode. All tests are carried out at a constant
 119 temperature of -10 °C.
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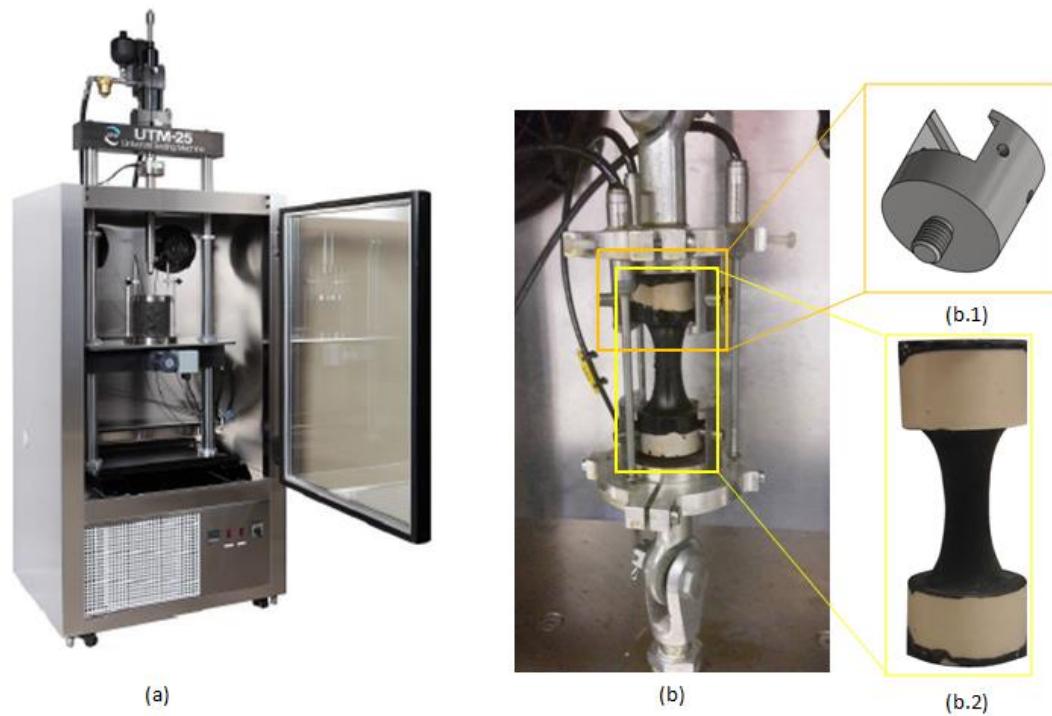


FIGURE 4 Universal Testing Machine UTM-25 (a), the frame (b) with modified hinges (b.1) and asphalt mortar specimen (b.2)

Induction Heating and Healing Performance

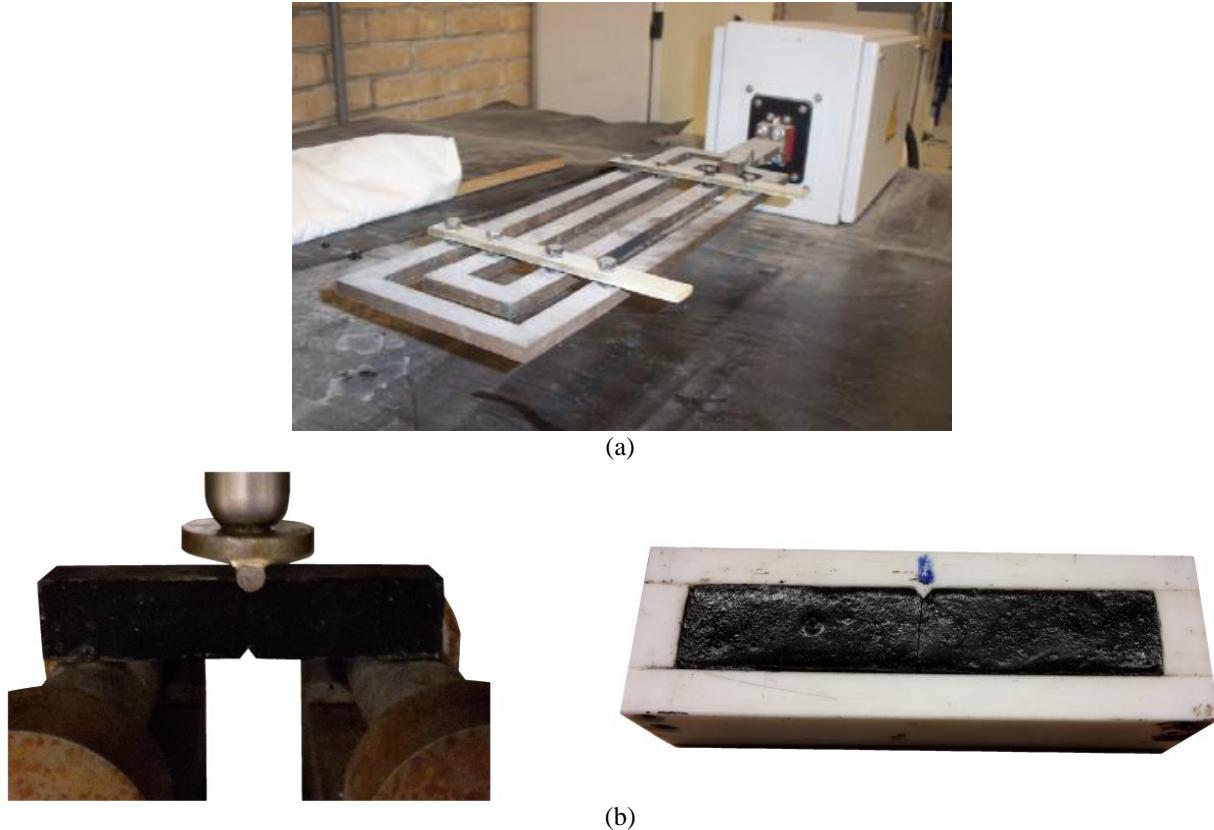
Among the objectives of this research is to determine the induction heating efficiency of the asphalt mortar with different combinations of additives. The induction heating experiment was performed with a 550 V RF generator 50/100 (Huttinger Electronic, Germany), see Figure 5.a, at a maximum frequency of 63.5 kHz. The distance from the mortar sample ($125 \times 20 \times 25$ mm) to the coil was 10 mm and the data were obtained from the surface of the specimen by using an infrared (IR) thermometer.

Additionally, in order to determine the healing efficiency of asphalt mortar after mixing conductive additives, asphalt mortar beams are produced with dimensions $105 \times 25 \times 13$ mm in a mould and with a notch at the middle, see Figure 5.b. A similar experimental procedure as proposed by Liu et al (8) was selected to test the healing capacity of the asphalt mortar. The sample is placed in a chamber at -10°C and is broken into two pieces using the three point bending setup, see Figure 5.b. The two pieces are then placed back into the mould. At the final stage, the two pieces are heated via induction energy until the surface temperature reaches 120°C . This process is continued after resting the sample for 2 hours at 20°C . Moreover, this process is repeated until the damage is too high to continue the healing process (8). Concerning the temperature, -10°C was chosen in order to avoid permanent deformation of the material and to obtain a brittle fractured surface. For the induction healing analysis, 5 samples were used for each type of conductive mortar.

The induction healing performance is evaluated by using the relation given in equation 4:

$$S(t) = \frac{F_i}{F_0} \quad (4)$$

147 where F_0 is the fracture force of the sample during a three point bending test, and F_i is the fracture force after
 148 the induction heating.
 149



150 **FIGURE 5 (a) Induction heating machine used at laboratory and (b) the three point bending setup with**
 151 **the asphalt mortar specimen used for the induction healing within mould**

152 **RESULTS**

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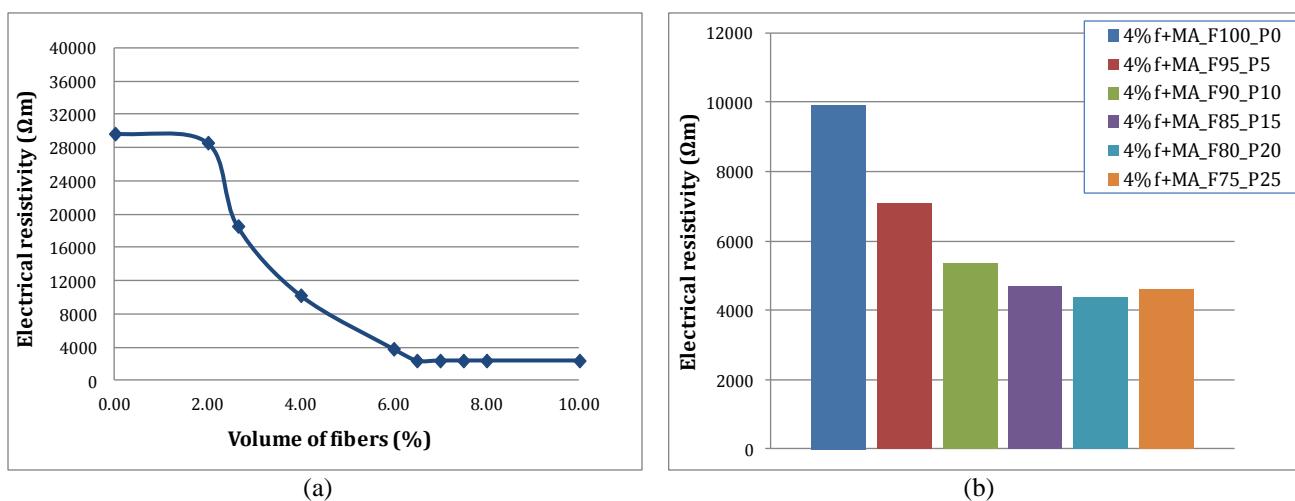
154 **Electrical Resistivity and Thermal Conductivity**

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156 The change of the electrical resistivity of an asphalt mortar with steel fibers, but without iron powder is
 157 shown in Figure 6.a. The conductive paths formed by steel fibers develop and lead to a gradual decrease of
 158 the resistivity above 2% volume of fibers. It is clear that the increase of the volume of steel fibers reduces the
 159 resistivity or increases the electrical conductivity of asphalt mortar. The optimum steel fibers content reached
 160 when no longer increases the electrical conductivity by adding more than 6.4% of steel fibers. For adding iron
 161 powder in the mortars with constant steel fibers content, it was selected asphalt mortar with 4% of steel fibers
 162 as a conductive mortar with amount of steel fibers beyond the percolation threshold.
 163

164 The combination of steel fibers and iron powder further improves considerably the electrical conductivity
 165 of the asphalt mortar, see Figure 6.b. It can be seen that, by choosing asphalt mortar with 4% of steel fibers
 166 and adding the iron powder stepwise in parallel with the reduction of mineral filler, the replacement of
 167 mineral filler with iron powder decreases the electrical resistivity of the asphalt mortar further. The optimum
 168 combination of additives in the asphalt mortar is 4% of steel fibers and 15% of iron powder. This

169 combination leads to a shorter conductive pathway in the mortar and hence the electrical resistivity of the
 170 asphalt mortar decreases significantly. This volume combination of steel fiber and iron powder will be used
 171 for the further steps of this research.
 172



173 **FIGURE 6 Effect of (a) the volume content of steel fibers and of (b) iron powder after substituting
 174 mineral filler with iron on the electrical resistivity of asphalt mortars**

175 For composite materials such as asphalt mixture, the thermal properties can be determined by the
 176 properties, dispersion and proportion of individual components in the final mix. By increasing the proportion
 177 of a component in the mix, the thermal properties of the final mix can be increased or decreased depending on
 178 the type and the nature of the component. An asphalt mixture can be considered as a combination of the
 179 components mortar and stone fraction. In this study, CTherm Analyzed was used to examine the thermal
 180 conductivity of the conductive asphalt mortars.
 181

182 It is observed that adding steel fibers to the asphalt mortar leads to increase of thermal conductivity, see
 183 Figure 7. Because of the thermal conductivity of steel fiber is quite high, when the volumetric part of steel
 184 fibers into the asphalt mortar is increased or decreased, the thermal properties of the whole mix will increase
 185 or decrease respectively. The increase of thermal conductivity is slightly higher in the case of asphalt mortars
 186 mixed with both iron powder and steel fibers.
 187

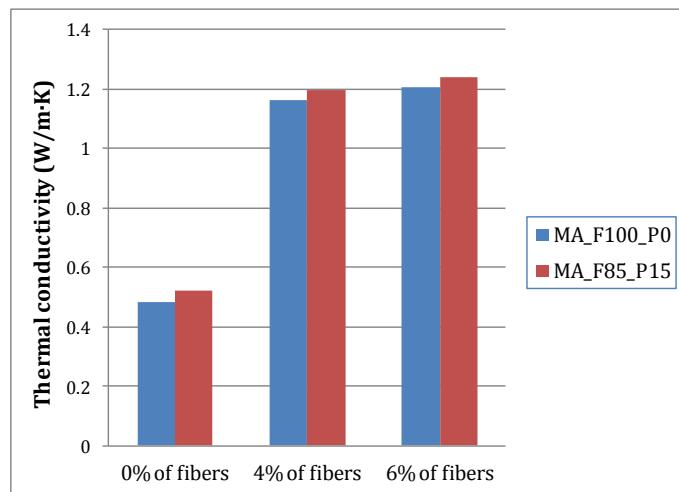


FIGURE 7 Effect of the volume content of steel fibers on the thermal conductivity of asphalt mortar with and without substituting mineral filler with iron powder

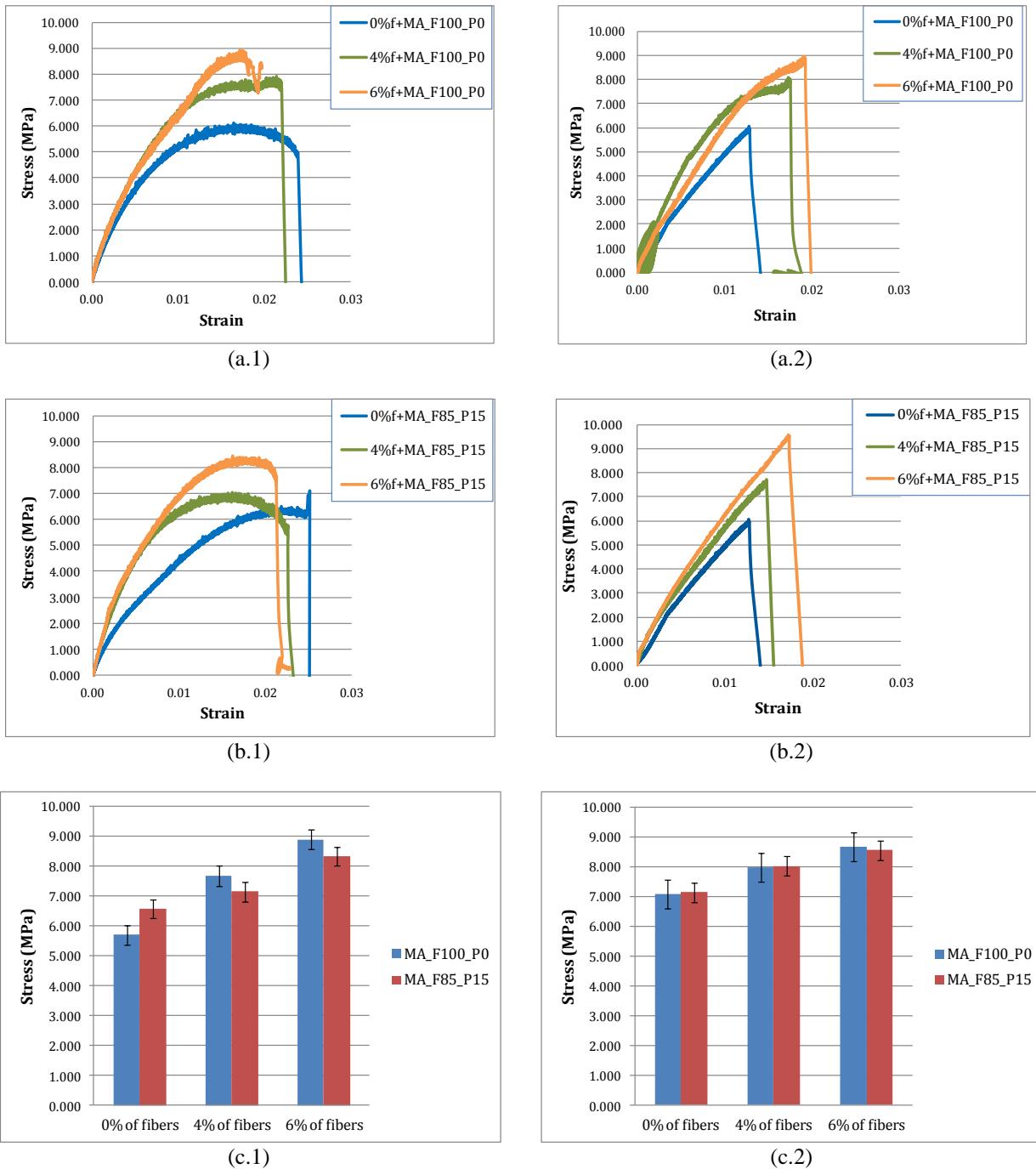
Tensile Strength and Fatigue Performance

The direct tensile strength and fatigue tests provide crucial information about the impact of additives on the mechanical performance of the conductive asphalt mortar. The asphalt mortar is the first decentralized system of an asphalt mixture and represents the matrix of the asphalt mixture between the aggregates. This implies that the mechanical behaviour of the mortar has a direct effect on the behaviour of the asphalt mixture on roads. The typical stress-strain curves at low temperatures (-10°C) and at different displacement rates are presented in Figures 8. It is obvious that the amount of steel fibres influences the maximum tensile stress. The tensile strength of the asphalt mortar increases with increasing fibre content. Therefore, the reinforcing effect of fibres on the asphalt mortar is apparent in Figure 8.c, where the average values of the maximum tensile stresses are presented.

The effect on brittleness and ductility of the conductive asphalt mortar can be observed in Figure 8. At high displacement rates, all samples show brittle response. More ductility can be observed for lower fiber contents and lower displacement rate. Particularly, the replacement of the part of mineral filler with iron powder, it did not influence significantly on the tensile strength of the asphalt mortar and the reinforcing effect of the fibers.

In order to study the fatigue response of asphalt mortar with different combinations of conductive additives, the cyclic sinusoidal load is utilized. The magnitude of the loading is defined as the 40% of the ultimate tensile strength (0.3 kN). The loading frequency was 5 Hz. and all the tests were carried out at -10 °C.

It can be observed that all the asphalt mortar samples show the tertiary phase of deformation after certain loading time, see Figures 9.a. and 9.b. Particularly, by increasing the amount of steel fibers within the asphalt mortar from 0% to 4%, the tertiary phase is significantly delayed and the fatigue life increases. Moreover, the fatigue life is extended when steel fibers were added from 4% to 6% within the asphalt mortar. It can be seen that the asphalt mortar with 15 % of iron powder appear slightly higher fatigue life than the one without iron powder, see Figure 9.c.



220
221 **FIGURE 8 Stress-strain curves for asphalt mortars; with mastic MA_F100_P0 and different amounts**
222 **of fibers, (a.1) displacement rate: 0.0275 mm/s and (a.2) 0.05 mm/s; with mastic MA_F85_P15 and**
223 **different amounts of fibers, (b.1) displacement rate: 0.0275 mm/s and (b.2) 0.05 mm/s; and the total**
224 **graphs with the tensile strength of asphalt mortars: displacement rate (c.1) 0.0275 mm/s and (c.2) 0.05**
225 **mm/s**

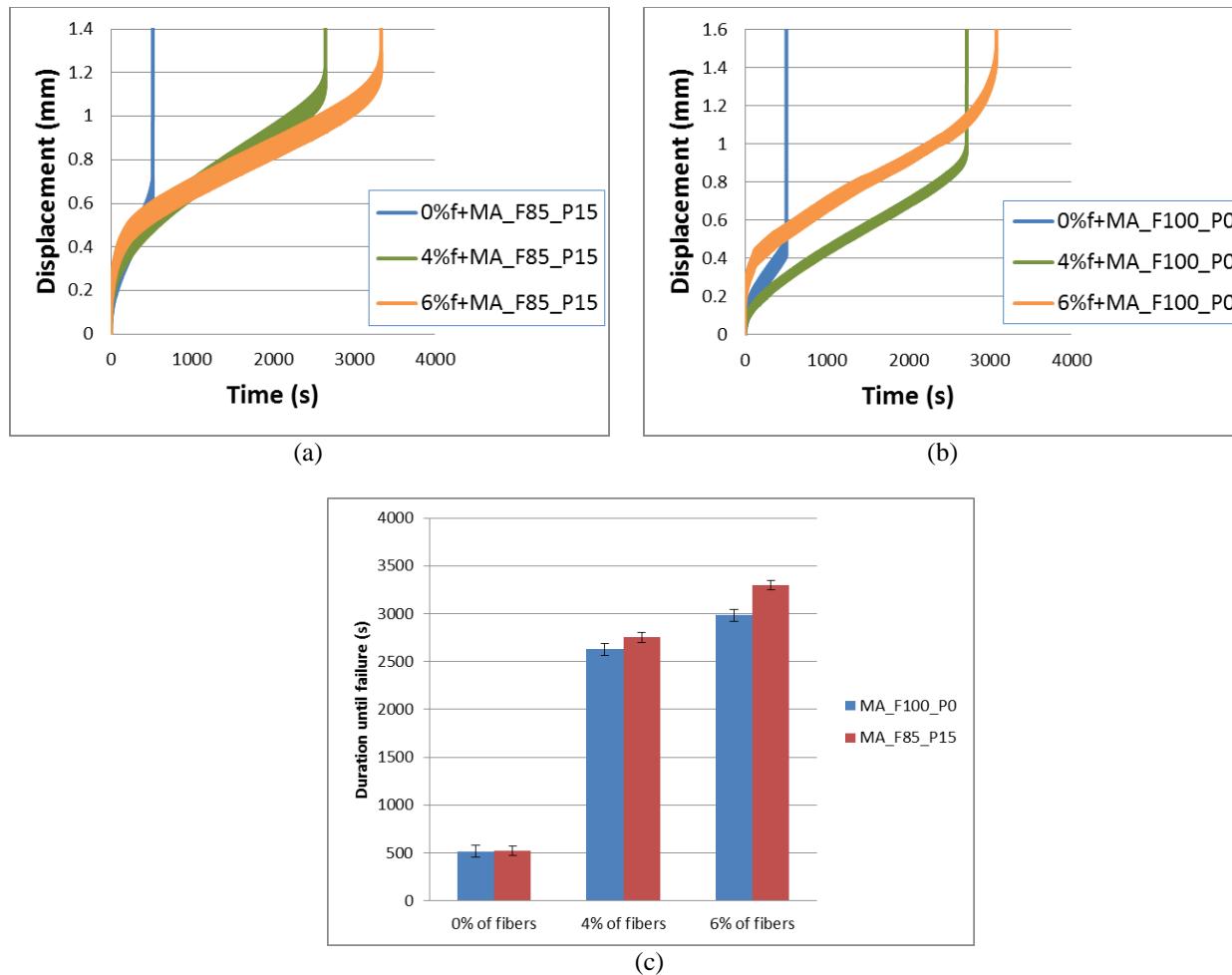


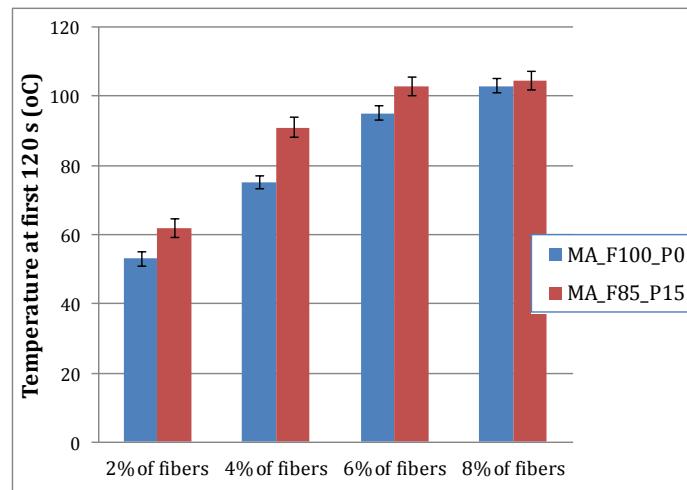
FIGURE 9 Influence of steel fibres on fatigue performance of asphalt mortars (a) with and (b) without iron powder, and (c) the total graph with the fatigue life of different mortars

Induction Heating and Healing Performance

In order to investigate the induction heating efficiency of the conductive asphalt mortar, at ambient temperature (20 °C), the test samples were heated for 120 seconds by inductor. The test samples were mixed with different volumetric combinations of steel fibers and iron powder. Figure 10 presents that the average temperature at the top surface of samples at 120 seconds induction heating. It can be observed that the maximum surface temperature is related to the volume of steel fibers added in the asphalt mortar. The higher amount of fibers in the mortar sample led to the higher surface temperature and hence the higher induction heating efficiency of the asphalt mortar. However, the tendency of increasing heating efficiency of the mortar is not linear increase. For example, after 6% of fibers added in the mortar, the tendency of increasing temperature is not significant and it is stabilized. It means that the mortars achieve the induction heating saturation limit where all the conductive paths are linked.

Similar observation can be found for the samples mixed with both iron powder and steel fibers. It can be seen that the induction heating efficiency can be enhanced by combination of iron powder and steel fibers

243 into the asphalt mortar. The average surface temperature of the samples with 15% iron powder is higher than
 244 the samples without powder.
 245



246
 247
 248 **FIGURE 10 Temperature reached after 120 seconds induction heating for asphalt mortar with**
 249 **constant volume of steel fibers and different volumes of iron powder**

250 The induction healing efficiency of asphalt mortar with steel fibers is presented in Figure 11.a. The cracks
 251 were healed by induction heating. However, after the first healing cycle, the strength was recovered by 60%
 252 of its original strength. This phenomenon can be explained by the loss of reinforcing effect of steel fibers in
 253 mortar (12). Apart from the induction healing of asphalt mortar, the use of steel fibers offers a reinforcing
 254 matrix with a network of random oriented fibers. However, when mortar is fractured, the interconnection
 255 among the fibers at the cracked surfaces is lost and mechanical performance of conductive mortar is as a
 256 material without fibers. In the second and third cycles, the strength recovery remained approximately
 257 constant. In the fourth cycle, material lost its strength completely. After several fracture - healing cycles, the
 258 cracked surfaces of fractured mortars were covered mostly by asphalt binder without steel fibers. As a result,
 259 the diffusion of binder from the one side of surface to the other was prohibited and subsequently the closure
 260 of crack of asphalt mortar. The fracture - healing process was continued successive in six cycles. Similar to
 261 the case of mortar mixed with fibers, the combination of steel fibers and iron powder can provide the same
 262 induction healing capacity to the mortar, see Figure 11.b.

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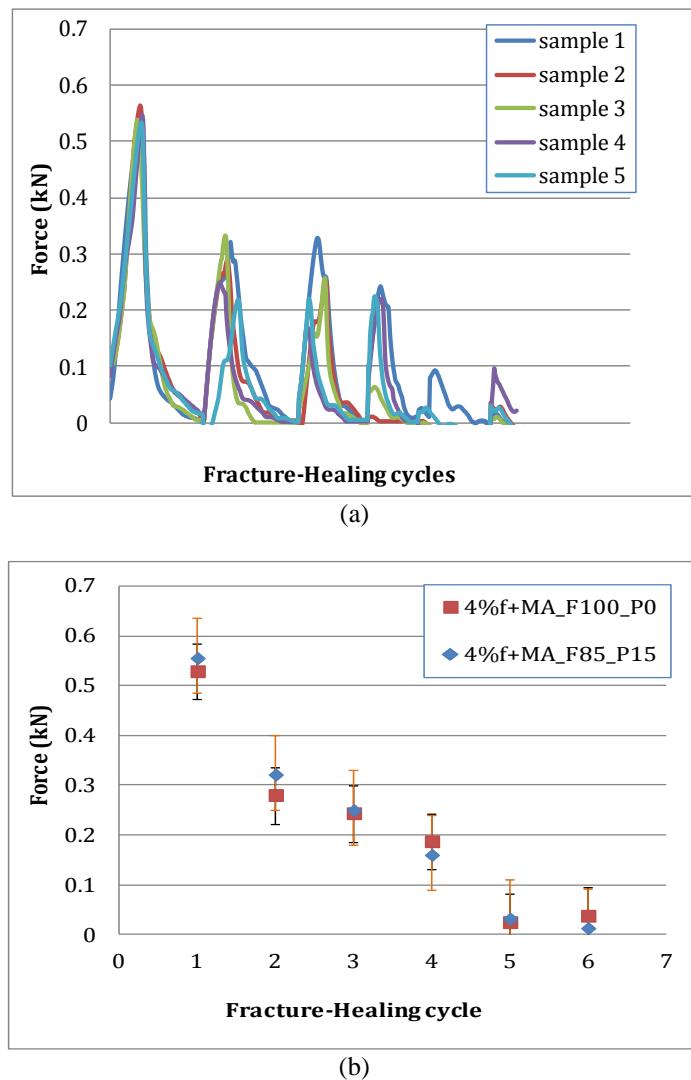


FIGURE 11 (a) Stress-strain curves for asphalt mortar containing 4% of steel fibers and (b) strength comparison for two types of asphalt mortars

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The findings of this research were within the efforts to enhance the induction heating of asphalt mixtures preparing simultaneously materials with improved mechanical performance during their service. Based on the results presented in this paper, the following conclusions can be made:

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1. The increase of conductive additives (e.g. iron powder and/or steel fibers) contributes to the enhancement of the electrical and thermal conductivity of asphalt mortar. The utilization of steel fibers has significant improvement on the electrical conductivity of asphalt mortar than the one with iron powder. Moreover,

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CONCLUSIONS

The findings of this research were within the efforts to enhance the induction heating of asphalt mixtures preparing simultaneously materials with improved mechanical performance during their service. Based on the results presented in this paper, the following conclusions can be made:

1. The increase of conductive additives (e.g. iron powder and/or steel fibers) contributes to the enhancement of the electrical and thermal conductivity of asphalt mortar. The utilization of steel fibers has significant improvement on the electrical conductivity of asphalt mortar than the one with iron powder. Moreover, combining steel fibers and iron powder within the asphalt mortar, the thermal conductivity is slightly higher than using only steel fibers as conductive additives.

- 279 2. When steel fibers are added in the asphalt mortar, the tensile strength is improved and the fatigue life is
280 extended. Similar mechanical response is obvious also by combining iron powder and steel fibers.
281 3. The induction heating efficiency is increased when iron powder and steel fibers are added to a certain
282 limit, where the temperature does not increase anymore, independently. Apart from the highest induction
283 heating efficiency, asphalt mortars have similar induction healing capacity with mortars with steel fibers
284 when iron powder is mixed.

285

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287

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289

290 **REFERENCES**

291

- 292 1. Bazin, P., J. Saunier. Deformation, fatigue and healing properties of asphalt mixes. *Proc., 2nd International Conference on the Structural Design of Asphalt Pavements*, Ann. Arbor, Mich., 1967, pp. 553-569.
- 293 2. Little, D.N., A. Bhasin. Exploring mechanisms of healing in asphalt mixtures and quantifying its impact. In *Self-Healing Materials: An Alternative Approach to 20 Centuries of Materials Science* (S. van de Zwaag, ed.), Springer Series in Materials Science, Vol. 100, Springer, Dordrecht, Netherlands, 2007, pp. 205-218.
- 294 3. Kim, B., R. Roque. Evaluation of healing property of asphalt mixtures. In *Transportation Reserach Record: Journal of the Transportation Research Board*, No. 1970, Trasportation Research Board of the National Academies, Washington, D.C., 2006, pp. 84-91.
- 295 4. Chan, S.B., B. Lane, T. Kazmierowski, W. Lee. Pavement preservation: a solution for sustainability. *Transportation Research Record: Journal of the Transportation Research Board*, No. 2235, Transportation Research Board of the National Academies, Washington, D.C., 2011, pp. 36-42.
- 296 5. Decker, D.S. *Best practices for crack treatment for asphalt pavement*, NCHRP Report 784 Transportation Research Board of the National Academies, Washington, D.C., 2014.
- 297 6. Garcia, A., E. Schlangen, M. van de Ven. Two ways of closing cracks on asphalt concrete pavement: microcapsules and induction heating. *Key Engineering Materials*, Vol. 417-418, 2010, pp 573-576.
- 298 7. Garcia, A., E. Schlangen, M. van de Ven, Q. Liu. Electrical conductivity of asphalt mastic containing conductive fibers and fillers. *Construction and Building Materials*, Vol. 23, 2009. pp. 3175-3181.
- 299 8. Liu, Q., A. Garcia, E. Schlangen, M. van de Ven. Induction healing of asphalt mastic and porous asphalt concrete. *Construction and Building Materials*, Vol. 25, 2011, pp. 3746-3752.
- 300 9. Liu, G., E. Schlangen, M. van de Ven. Induction healing of porous asphalt concrete. In *Transportation Research Record,: Journal of the Transportation Research Board*, No 2305, Transportation Research Board of the National Academies, Washington, D.C., 2012, pp. 95-101.
- 301 10. Garcia, A., J. Norambuena-Contreras, M.N. Partl. Experimental evaluation of dense asphalt concrete properties for induction heating purposes. *Construction and Building Materials*, 46, 2013, pp. 48-54.
- 302 11. Garcia A., J. Norambuena-Contreras, M.N. Partl, P. Schuetz. Uniformity and mechanical properties of dense asphalt concrete with steel wool fibers. *Construction and Building Materials*, Vol. 43, 2013, pp. 107-117.
- 303 12. Liu Q., S.Wu, E.Schlangen. Induction heating of asphalt mastic for crack control. *Construction and Building Materials*, Vol. 41, 2013, pp. 345-351.
- 304 13. Brown, SF., R.D. Rowlett, J.L. Boucher. Asphalt modification. In: *Proceedings of the conference on the United States Strategic Highway Research Program: Sharing the Benefits*. Thomas Telford, London,1990, pp 181-203.

- 326 14. Huang, H., T.D. White. Dynamic properties of fiber-modified overlay mixture. *Transportation Research*
327 *Record*, 1545, 1996, pp.98-104.
- 328 15. Wu, S., Q. Ye, N. Li, H. Yue. Effects of fibers on the dynamic properties of asphalt mixtures. *Journal of*
329 *Wuhan University Technology Mater. Sc. Ed.* 22, 2007, pp. 733-736.
- 330 16. Kaloush, K.E., K.P. Biligiri, W.A. Zeiada, M.C. Rodezno, J.X. Reed. Evaluation of fiber-reinforced asphalt
331 mixtures using advanced material characterization tests. *Journal of Testing and Evaluation*, Vol. 38, No. 4,
332 2010,
- 333 17. Punith, V.S., S.N. Suresha, A. Veeraragavan, S. Raju, S. Bose. Characterization of polymer and fiber-
334 modified porous asphalt mixtures. *TRB 2004 Annual Meeting CD-ROM*. *Transportation Research Board*.
335 *National Research Council*. Washington DC, 2004.
- 336 18. Zhong, Y.X., X.L. Wang, M.L. Liao. Experiment research on performance of low-temperature crack
337 resistance of reinforced asphalt mixtures. *Advanced Materials Research*, Vol. 163-167, 2010, pp. 1128-1133.
- 338 19. Kim, H., K. Sokolov, L.D. Poulikakos, M.N. Partl. Fatigue evaluation of porous asphalt composites with
339 carbon fiber reinforcement polymer grids. *Trasportation Research Record*, 2116, 2009, pp. 108-117.
- 340 20. Lee S.J., J.P. Rust, H. Hamouda, Y.R. Kim, R.H. Borden. Fatigue cracking resistance of fiber-reinforced
341 asphalt concrete. *Textile Research Journal*, 75, 2005, pp. 123-128.